#### **Stochastic Structural Dynamics**

#### Lecture-16

Random vibration analysis of MDOF systems-4

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#### VIBRATION ANALYSIS OF CONTINUOUS SYSTEMS

$$\frac{\partial^2}{\partial x^2} \left[ EI(x) \frac{\partial^2 y}{\partial x^2} + \varepsilon(x) \frac{\partial^3 y}{\partial x^2 \partial t} \right] + m(x) \ddot{y} + c(x) \dot{y} = f(x, t)$$
  
ICS:  $y_0(x) = y(x, 0)$   $\dot{y}_0(x) = \dot{y}(x, 0)$  & BCS as appropriate.  
 $\varepsilon(x) = v EI(x)$ 

$$y(x,t) = \sum_{n=1}^{\infty} a_n(t)\varphi_n(x)$$
  

$$\begin{bmatrix} EI\varphi_n'' \end{bmatrix}'' = m\omega_n^2\varphi_n(x)$$
  

$$\int_0^L EI\varphi_n''\varphi_k''dx = 0 \quad n \neq k \quad \int_0^L m\varphi_n\varphi_k dx = 0 \quad n \neq k$$

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$$\ddot{a}_n + 2\eta_n \omega_n \dot{a}_n + \omega_n^2 a_n = p_n(t);$$
  

$$2\eta_n \omega_n = (\alpha + \nu \omega_n^2);$$
  

$$p_n(t) = \frac{\int_0^L \varphi_n(x) f(x, t) dx}{\int_0^L \varphi_n^2(x) m(x) dx} \qquad n = 1, 2, \dots \infty$$

$$y(x,t) = \sum_{n=1}^{\infty} \phi_n(x) \left\{ \exp(-\eta_n \omega_n t) [A_n \cos \omega_{dn} t + B_n \sin \omega_{dn} t] + \int_0^t h_n(t-\tau) p_n(\tau) d\tau \right\}$$

•Displacement: 
$$y(x,t) = \sum_{n=1}^{N \to \infty} a_n(t) \phi_n(x)$$
  
•Slope:  $y'(x,t) = \sum_{n=1}^{N \to \infty} a_n(t) \phi'_n(x)$   
•Bending moment:  $EI(x) y''(x,t) = \sum_{n=1}^{N \to \infty} a_n(t) EI(x) \phi''_n(x)$   
•Shear force:  $\left[ EI(x) y''(x,t) \right]' = \sum_{n=1}^{N \to \infty} a_n(t) \left[ EI(x) \phi''_n(x) \right]'$   
Other quantities  
•Bending stress  
•Shear stress  
•Principal stresses

#### A clamped beam under differential support displacements

The BCS are time dependent.

Modal expansion method cannot be used directly.

Introduce a new dependent variable  

$$y(x,t) = w(x,t) + h_1(x)u(t) + h_2(x)v(t)$$

$$y(0,t) = w(0,t) + h_1(0)u(t) + h_2(0)v(t) = u(t)$$
Select  $w(0,t) = 0$ ;  $h_1(0) = 1$ ; &  $h_2(0) = 0$   
 $y'(0,t) = w'(0,t) + h'_1(0)u(t) + h'_2(0)v(t) = 0$   
Select  $w'(0,t) = 0$ ;  $h'_1(0) = 0$ ; &  $h'_2(0) = 0$   
 $y(l,t) = w(l,t) + h_1(l)u(t) + h_2(l)v(t) = v(t)$   
Select  $w(l,t) = 0$ ;  $h_1(l) = 0$ ; &  $h_2(l) = 1$   
 $y'(l,t) = w'(l,t) + h'_1(l)u(t) + h'_2(l)v(t) = 0$   
Select  $w'(l,t) = 0$ ;  $h'_1(l) = 0$ ; &  $h'_2(l) = 0$ 

$$EI\left[w^{iv} + h_{1}^{iv}u + h_{2}^{iv}u\right] + m\left[\ddot{w} + h_{1}\ddot{u} + h_{2}\ddot{v}\right] + c\left[\dot{w} + h_{1}\dot{u} + h_{2}\dot{v}\right] = 0$$
Select
$$h_{1}^{iv} = 0$$

$$h_{2}^{iv} = 0$$

$$h_{1}(x) = ax^{3} + bx^{2} + cx + d$$

$$h_{1}(0) = 1; h_{1}(l) = 0; h_{1}'(0) = 0; h_{1}'(l) = 0;$$

$$h_{1}(x) = 1 - \frac{3x^{2}}{L^{2}} + \frac{2x^{3}}{L^{3}} / /$$
Similarly
$$h_{2}(x) = \frac{3x^{2}}{L^{2}} - \frac{2x^{3}}{L^{3}}$$



$$\begin{split} EI\left[w^{iv} + h_1^{iv}u + h_2^{iv}u\right] + m\left[\ddot{w} + h_1\ddot{u} + h_2\ddot{v}\right] + c\left[\dot{w} + h_1\dot{u} + h_2\dot{v}\right] = 0\\ \Rightarrow EIw^{iv} + m\ddot{w} + c\dot{w} = \\ -m\left[\ddot{u}\left(1 - \frac{3x^2}{L^2} + \frac{2x^3}{L^3}\right) + \ddot{v}\left(\frac{3x^2}{L^2} - \frac{2x^3}{L^3}\right)\right]\\ -c\left[\dot{u}\left(1 - \frac{3x^2}{L^2} + \frac{2x^3}{L^3}\right) + \dot{v}\left(\frac{3x^2}{L^2} - \frac{2x^3}{L^3}\right)\right] = f(x,t)\\ w(0,t) = 0; w'(0,t) = 0\\ w(l,t) = 0; w'(l,t) = 0\\ w(x,0) = -h_1(x)u(0) - h_2(x)v(0)\\ w(x,0) = -h_1(x)\dot{u}(0) - h_2(x)\dot{v}(0) \end{split}$$

Eigenfunction expansion method can now be used.

$$u \oplus \int \frac{1}{\sqrt{y_{(2,+)}}} \int v(t) dt = w(x,t) + [h_1(x)u(t) + h_2(x)v(t)]$$



Harmonically driven beam: Green's functions in frequency domain



$$\frac{\partial^2}{\partial x^2} \left[ EI(x) \frac{\partial^2 y}{\partial x^2} + vEI(x) \frac{\partial^3 y}{\partial x^2 \partial t} \right] + m(x) \ddot{y} + c(x) \dot{y} = \exp(i\omega t) \delta(x - \xi)$$
  
ICS:  $y_0(x) = y(x, 0)$   $\dot{y}_0(x) = \dot{y}(x, 0)$   
BCS:  $y(0, t) = 0$ ;  $EIy''(0, t) = 0$ ;  $y(L, t) = 0$ ;  $EIy''(L, t) = 0$ 

$$\lim_{t\to\infty} y(x,t) = ?$$

$$y(x,t) = \sum_{n=1}^{\infty} a_n(t)\varphi_n(x)$$
$$\begin{bmatrix} EI\varphi_n'' \end{bmatrix}'' = m\omega_n^2\varphi_n(x)$$
$$\int_0^L EI\varphi_n''\varphi_k''dx = 0 \quad n \neq k \quad \int_0^L m\varphi_n\varphi_k dx = 0 \quad n \neq k$$

$$\ddot{a}_{n} + 2\eta_{n}\omega_{n}\dot{a}_{n} + \omega_{n}^{2}a_{n} = \int_{0}^{L} \exp(i\omega t)\phi_{n}(x)\delta(x-\xi)dx$$
$$= \phi_{n}(\xi)\exp(i\omega t);$$
$$n = 1, 2, \dots \infty$$

$$\begin{split} \lim_{t \to \infty} a_n(t) &\to \frac{\phi_n(\xi) \exp(i\omega t)}{\omega_n^2 - \omega^2 + i2\eta_n \omega \omega_n} \\ \Rightarrow \\ \lim_{t \to \infty} y(x,t) &= \sum_{n=1}^{N \to \infty} \frac{\phi_n(x)\phi_n(\xi) \exp(i\omega t)}{\omega_n^2 - \omega^2 + i2\eta_n \omega \omega_n} \\ &= G(x,\xi,\omega) \exp(i\omega t) \\ \text{with} \\ G(x,\xi,\omega) &= \sum_{n=1}^{N \to \infty} \frac{\phi_n(x)\phi_n(\xi)}{\omega_n^2 - \omega^2 + i2\eta_n \omega \omega_n} \\ G(x,\xi,\omega) &= \text{Green's function} \end{split}$$

#### Note

- • $G(x,\xi,\omega) = G(\xi,x,\omega)$ • $G(x,\xi,\omega)$  is complex valued • $G(x,\xi,\omega)$  is the generalization of the FRF discussed earlier

$$H_{ij}(\omega)$$
  
 $x, \xi$ 







Response of beam to a general load *f(t)* Note: the Fourier transform of *f(t)* is taken to exist



$$\frac{\partial^{2}}{\partial x^{2}} \left[ EI(x) \frac{\partial^{2} y}{\partial x^{2}} + v EI(x) \frac{\partial^{3} y}{\partial x^{2} \partial t} \right] + m(x) \ddot{y} + c(x) \dot{y} = f(t) \delta(x - \xi)$$
ICS:  $y_{0}(x) = y(x, 0)$   $\dot{y}_{0}(x) = \dot{y}(x, 0)$ 
BCS:  $y(0, t) = 0$ ;  $EIy''(0, t) = 0$ ;  $y(L, t) = 0$ ;  $EIy''(L, t) = 0$ 

$$\chi(\omega) = H^{(\omega)} F^{(\omega)}$$

$$Y(x, \omega) = G(x, \xi, \omega) F(\omega)$$

Beam driven by impulse excitation: Green's functions in time domain



$$\frac{\partial^2}{\partial x^2} \left[ EI(x) \frac{\partial^2 y}{\partial x^2} + v EI(x) \frac{\partial^3 y}{\partial x^2 \partial t} \right] + m(x) \ddot{y} + c(x) \dot{y} = \underbrace{\delta(t-0)\delta(x-\xi)}_{\text{ICS:}} y_0(x) = y(x,0) = 0 \quad \dot{y}_0(x) = \dot{y}(x,0) = 0 \quad \text{if } x = 0$$
  
BCS:  $y(0,t) = 0$ ;  $EIy''(0,t) = 0$ ;  $y(L,t) = 0$ ;  $EIy''(L,t) = 0$ 

$$y(x,t) = \sum_{n=1}^{\infty} a_n(t)\varphi_n(x)$$
$$\begin{bmatrix} EI\varphi_n'' \end{bmatrix}'' = m\omega_n^2\varphi_n(x)$$
$$\int_0^L EI\varphi_n''\varphi_k''dx = 0 \quad n \neq k \quad \int_0^L m\varphi_n\varphi_k dx = 0 \quad n \neq k$$

$$\ddot{a}_{n} + 2\eta_{n}\omega_{n}\dot{a}_{n} + \omega_{n}^{2}a_{n} = \int_{0}^{L}\phi_{n}(x)\delta(t)\delta(x-\xi)dx$$
$$= \phi_{n}(\xi)\delta(t);$$
$$n = 1, 2, \dots \infty$$

$$g(x,\xi,t) = g(\xi,x,t)$$







#### Response of beam to a concentrated load f(t)

$$\frac{\int \frac{\partial^2}{\partial x^2} \left[ EI(x) \frac{\partial^2 y}{\partial x^2} + v EI(x) \frac{\partial^3 y}{\partial x^2 \partial t} \right] + m(x) \ddot{y} + c(x) \dot{y} = f(t) \delta(x - \xi)$$

$$ICS: \quad y_0(x) = y(x, 0) = 0 \quad \dot{y}_0(x) = \dot{y}(x, 0) = 0$$

$$BCS: y(0, t) = 0; EIy''(0, t) = 0; y(L, t) = 0; EIy''(L, t) = 0$$

$$hij \qquad y(x, t) = \int_0^t g(x, \xi, t - \tau) f(\tau) d\tau$$

# **Exercise** Show that $g(x,\xi,t) \Leftrightarrow G(x,\xi,\omega) \qquad h(t) \rightleftharpoons H(\omega)$ That is $G(x,\xi,\omega) = \int_{-\infty}^{\infty} g(x,\xi,t) \exp(i\omega t) dt$ $g(x,\xi,t) = \frac{1}{2\pi} \int_{-\infty}^{\infty} G(x,\xi,\omega) \exp(-i\omega t) d\omega$

Response of beam to a general load f(x,t)



$$\frac{\partial^2}{\partial x^2} \left[ EI(x) \frac{\partial^2 y}{\partial x^2} + v EI(x) \frac{\partial^3 y}{\partial x^2 \partial t} \right] + m(x) \ddot{y} + c(x) \dot{y} = f(x,t)$$
  
ICS:  $y_0(x) = y(x,0) = 0$   $\dot{y}_0(x) = \dot{y}(x,0) = 0$   
BCS:  $y(0,t) = 0$ ;  $EIy''(0,t) = 0$ ;  $y(L,t) = 0$ ;  $EIy''(L,t) = 0$ 

$$y(x,t) = \int_{0}^{L} \int_{0}^{t} g(x,\xi,t-\tau)f(\xi,\tau)d\xi d\tau$$
$$Y(x,\omega) = \int_{0}^{L} G(x,\xi,\omega)F(\xi,\omega)d\xi$$

#### Response of beam to a concentrated random load f(t)



$$\frac{\partial^2}{\partial x^2} \left[ EI(x) \frac{\partial^2 y}{\partial x^2} + v EI(x) \frac{\partial^3 y}{\partial x^2 \partial t} \right] + m(x) \ddot{y} + c(x) \dot{y} = \underline{f(t)} \delta(x - \xi)$$

$$\underbrace{\langle f(t) \rangle}_{\text{ICS:}} = 0; \underbrace{\langle f(t) f(t + \tau) \rangle}_{y_0(x) = y(x,0) = 0} = R_{ff}(\tau) \underbrace{\langle f(t) \rangle}_{y_0(x) = y(x,0) = 0} = 0$$

$$BCS: y(0,t) = 0; EIy''(0,t) = 0; y(L,t) = 0; EIy''(L,t) = 0$$

$$y(x,t) = \int_{0}^{t} g(x,\xi,t-\tau)f(\tau)d\tau$$

$$\langle y(x,t) \rangle = \int_{0}^{t} g(x,\xi,t-\tau) \langle f(\tau) \rangle d\tau = 0$$

$$\langle y(x_{1},t_{1}) y(x_{2},t_{2}) \rangle = \int_{0}^{t_{1}t_{2}} g(x_{1},\xi,t_{1}-\tau_{1}) g(x_{2},\xi,t_{2}-\tau_{2}) \langle f(\tau_{1}) f(\tau_{2}) \rangle d\tau_{1} d\tau_{2}$$

$$= \int_{0}^{t_{1}t_{2}} g(x_{1},\xi,t_{1}-\tau_{1}) g(x_{2},\xi,t_{2}-\tau_{2}) R_{ff}(\tau_{1},\tau_{2}) d\tau_{1} d\tau_{2}$$

$$= \int_{0}^{t_{1}t_{2}} g(x_{1},\xi,t_{1}-\tau_{1}) g(x_{2},\xi,t_{2}-\tau_{2}) R_{ff}(\tau_{1}-\tau_{2}) d\tau_{1} d\tau_{2}$$

$$\left\langle y(x_{1},t_{1}) y(x_{2},t_{2}) \right\rangle = \int_{0}^{t_{1}t_{2}} g(x_{1},\xi,t_{1}-\tau_{1}) g(x_{2},\xi,t_{2}-\tau_{2}) R_{ff}(\tau_{1}-\tau_{2}) d\tau_{1} d\tau_{2}$$

$$= \int_{0}^{t_{1}t_{2}} g(x_{1},\xi,t_{1}-\tau_{1}) g(x_{2},\xi,t_{2}-\tau_{2}) \left[ \frac{1}{\pi} \int_{0}^{\infty} S_{ff}(\omega) \cos \omega(\tau_{1}-\tau_{2}) d\omega \right] d\tau_{1} d\tau_{2}$$

$$= \int_{0}^{\infty} S_{ff}(\omega) \mathcal{W}(x_{1},x_{2},\xi,t_{1},t_{2},\omega) d\omega / /$$

$$\mathcal{W}(x_{1},x_{2},\xi,t_{1},t_{2},\omega) = \int_{0}^{t_{1}t_{2}} g(x_{1},\xi,t_{1}-\tau_{1}) g(x_{2},\xi,t_{2}-\tau_{2}) \cos \omega(\tau_{1}-\tau_{2}) d\tau_{1} d\tau_{2}$$

$$\left\langle y^{2}(x,t) \right\rangle = \int_{0}^{\infty} S_{ff}(\omega) \mathcal{W}(x,x,\xi,t,t,\omega) d\omega$$

Steady state response  

$$Y_{T}(x,\xi,\omega) = G(x,\xi,\omega) F_{T}(\omega)$$

$$S_{YY}(x,\xi,\omega) = \lim_{T \to \infty} \frac{1}{T} \left\langle \left| Y_{T}(x,\xi,\omega) \right|^{2} \right\rangle$$

$$= \left| G(x,\xi,\omega) \right|^{2} S_{FF}(\omega)$$

Beam excited by space-time white noise forcing (Rain on the roof excitation)



$$\frac{\partial^2}{\partial x^2} \left[ EI(x) \frac{\partial^2 y}{\partial x^2} + vEI(x) \frac{\partial^3 y}{\partial x^2 \partial t} \right] + \underbrace{m(x) \ddot{y} + c(x) \dot{y} = f(x,t)}_{=} \left\{ \frac{f(x,t)}{\partial x^2} = 0; \left\langle f(x,t) f(x+\xi,t+\tau) \right\rangle = I_0 \underbrace{m(x) \delta(\xi) \delta(\tau)}_{=} \right\}$$

$$\frac{\left\langle f(x,t) \right\rangle}{ICS: \quad y_0(x) = y(x,0) = 0 \quad \dot{y}_0(x) = \dot{y}(x,0) = 0}$$

$$BCS: y(0,t) = 0; EIy''(0,t) = 0; y(L,t) = 0; EIy''(L,t) = 0$$

$$y(x,t) = \sum_{n=1}^{\infty} a_n(t)\varphi_n(x)$$
$$\begin{bmatrix} EI\varphi_n'' \end{bmatrix}'' = m\omega_n^2\varphi_n(x)$$
$$\int_0^L EI\varphi_n''\varphi_k''dx = 0 \quad n \neq k \quad \int_0^L m\varphi_n\varphi_k dx = 0 \quad n \neq k$$

$$\ddot{a}_n + 2\eta_n \omega_n \dot{a}_n + \omega_n^2 a_n = \int_0^L \phi_n(x) f(x,t) dx; n = 1, 2, \dots \infty$$
$$a_n(t) = \int_0^t \int_0^L h_n(t-\tau) \phi_n(x) f(x,\tau) dx d\tau$$

$$a_{n}(t) = \int_{0}^{t} \int_{0}^{L} h_{n}(t-\tau) \phi_{n}(x) f(x,\tau) dx d\tau$$

$$\langle a_{n}(t) \rangle = \int_{0}^{t} \int_{0}^{L} h_{n}(t-\tau) \phi_{n}(x) \langle f(x,\tau) \rangle dx d\tau = 0$$

$$\langle a_{n}(t_{1}) a_{k}(t_{2}) \rangle = \int_{0}^{t_{1}} \int_{0}^{t_{2}} \int_{0}^{L} h_{n}(t_{1}-\tau_{1}) \phi_{n}(x_{1}) h_{k}(t_{2}-\tau_{2}) \phi_{k}(x_{2})$$

$$\langle f(x_{1},\tau_{1}) f(x_{2},\tau_{2}) \rangle dx_{1} dx_{2} d\tau_{1} d\tau_{2}$$

$$= \int_{0}^{t_{1}} \int_{0}^{t_{2}} \int_{0}^{L} h_{n}(t_{1}-\tau_{1}) \phi_{n}(x_{1}) h_{k}(t_{2}-\tau_{2}) \phi_{k}(x_{2})$$

$$I_{0}m(x_{1}) \delta(\tau_{1}-\tau_{2}) \delta(x_{1}-x_{2}) dx_{1} dx_{2} d\tau_{1} d\tau_{2}$$

$$y(x,t) = \sum_{n=1}^{N \to \infty} a_n(t) \phi_n(x)$$

$$\Rightarrow$$

$$\langle y(x,t) \rangle = \sum_{n=1}^{N \to \infty} \phi_n(x) \langle a_n(t) \rangle = 0$$

$$\langle y(x_1,t_1) y(x_2,t_2) \rangle = \sum_{n=1}^{N \to \infty} \sum_{k=1}^{N \to \infty} \phi_n(x_1) \phi_k(x_2) \langle a_n(t_1) a_k(t_2) \rangle$$

$$= \sum_{n=1}^{N \to \infty} \phi_n(x_1) \phi_n(x_2) \langle a_n(t_1) a_n(t_2) \rangle$$

$$\Rightarrow$$

$$\langle y^2(x,t) \rangle = \sum_{n=1}^{N \to \infty} \langle a_n^2(t) \rangle \phi_n^2(x)$$

Beam excited by a space-time random process



$$EI \frac{\partial^{4} y}{\partial x^{4}} + \underline{m}\ddot{y} + \underline{c}\dot{y} = f(x,t)$$

$$(f(x,t)) = 0; \langle f(x,t) f(x+\xi,t+\tau) \rangle = I_{0}\delta(\xi)R(\tau)$$

$$ICS: y_{0}(x) = y(x,0) = 0 \quad \dot{y}_{0}(x) = \dot{y}(x,0) = 0$$

$$BCS: y(0,t) = 0; EIy''(0,t) = 0; y(L,t) = 0; EIy''(L,t) = 0$$

$$y(x,t) = \sum_{n=1}^{\infty} a_n(t)\phi_n(x)$$
  
=  $\sum_{n=1}^{\infty} \phi_n(x) \int_{0}^{t} \int_{0}^{L} h_n(t-\tau)\phi_n(s) f(s,\tau) ds d\tau$   
 $\Rightarrow \langle y(x,t) \rangle = \sum_{n=1}^{\infty} \phi_n(x) \int_{0}^{t} \int_{0}^{L} h_n(t-\tau)\phi_n(s) \langle f(s,\tau) \rangle ds d\tau = 0$   
 $\langle y(x_1,t_1) y(x_2,t_2) \rangle = \sum_{n=1}^{\infty} \sum_{k=1}^{\infty} \phi_n(x_1) \phi_k(x_2) \int_{0}^{t_1} \int_{0}^{t_2} \int_{0}^{L} h_n(t_1-\tau_1) h_k(t_2-\tau_2)$   
 $\phi_n(s_1) \phi_k(s_2) \langle f(s_1,\tau_1) f(s_2,\tau_2) \rangle ds_1 ds_2 d\tau_1 d\tau_2$   
=  $\sum_{n=1}^{\infty} \sum_{k=1}^{\infty} \phi_n(x_1) \phi_k(x_2) \int_{0}^{t_1} \int_{0}^{t_2} \int_{0}^{L} h_n(t_1-\tau_1) h_k(t_2-\tau_2)$   
 $\phi_n(s_1) \phi_k(s_2) \delta(s_1-s_2) R(\tau_1-\tau_2) ds_1 ds_2 d\tau_1 d\tau_2$ 

$$\left\langle y(x_{1},t_{1}) y(x_{2},t_{2}) \right\rangle = \sum_{n=1}^{\infty} \sum_{k=1}^{\infty} \phi_{n}(x_{1}) \phi_{k}(x_{2}) \int_{0}^{t_{1}} \int_{0}^{t_{2}} \int_{0}^{L} \int_{0}^{L} h_{n}(t_{1}-\tau_{1}) h_{k}(t_{2}-\tau_{2}) \phi_{n}(x_{2}) \phi_{n}(x_{2}) \delta(x_{1}-x_{2}) dx_{1} dx_{2} d\tau_{1} d\tau_{2}$$

$$= \sum_{n=1}^{\infty} \sum_{k=1}^{\infty} \phi_{n}(x_{1}) \phi_{k}(x_{2}) \int_{0}^{t_{1}} \int_{0}^{t_{2}} \int_{0}^{L} h_{n}(t_{1}-\tau_{1}) h_{k}(t_{2}-\tau_{2}) \phi_{n}(x_{2}) \phi_{k}(x_{2}) R(\tau_{1}-\tau_{2}) dx_{2} d\tau_{1} d\tau_{2}$$
Recall
$$\int_{0}^{L} m \phi_{n} \phi_{k} dx = \delta_{nk} \Rightarrow // C \quad \text{constant}$$

$$\left\langle y(x_{1},t_{1}) y(x_{2},t_{2}) \right\rangle = \sum_{n=1}^{\infty} \phi_{n}(x_{1}) \phi_{n}(x_{2}) (1/m) \int_{0}^{t_{1}} \int_{0}^{t_{2}} h_{n}(t_{1}-\tau_{1}) h_{n}(t_{2}-\tau_{2}) R(\tau_{1}-\tau_{2}) d\tau_{1} d\tau_{2}$$

#### Beam under random support motions



$$EIy^{iv} + m\ddot{y} + c\dot{y} = 0$$

$$y(0,t) = 0; y'(0,t) = 0$$

$$y(l,t) = v(t); y'(l,t) = 0$$

$$y(x,0) = 0; \dot{y}(x,0) = 0$$

$$\langle v(t) \rangle = 0$$

$$\langle v(t) v(t+\tau) \rangle = R_{vv}(\tau) \Leftrightarrow S_{vv}(\omega)$$

Introduce a new dependent variable  

$$y(x,t) = w(x,t) + h(x)v(t)$$
  
 $y(0,t) = w(0,t) + h(0)v(t) = 0$   
Select  $w(0,t) = 0$ ;  $h(0) = 0$   
 $y'(0,t) = w'(0,t) + h'(0)v(t) = 0$   
Select  $w'(0,t) = 0$ ;  $h'(0) = 0$   
 $y(l,t) = w(l,t) + h(l)v(t) = v(t)$   
Select  $w(l,t) = 0$ ;  $h(l) = 1$   
 $y'(l,t) = w'(l,t) + h'(l)v(t) = 0$   
Select  $w'(l,t) = 0$ ;  $h'(l) = 0$ 

$$EI\left[w^{iv} + h^{iv}v\right] + m\left[\ddot{w} + h\ddot{v}\right] + c\left[\dot{w} + h\dot{v}\right] = 0$$
  
Select  
$$h^{iv} = 0$$
  
$$h(x) = ax^{3} + bx^{2} + cx + d$$
  
$$h(0) = 0; h(l) = 1; h'(0) = 0; h'(l) = 0;$$
  
$$h(x) = \frac{3x^{2}}{L^{2}} - \frac{2x^{3}}{L^{3}}$$

$$EI\left[w^{iv} + h^{iv}v\right] + m\left[\ddot{w} + h\ddot{v}\right] + c\left[\dot{w} + h\dot{v}\right] = 0$$
  

$$\Rightarrow EIw^{iv} + m\ddot{w} + c\dot{w} =$$
  

$$-m\left[\ddot{v}\left(\frac{3x^2}{L^2} - \frac{2x^3}{L^3}\right)\right] - c\left[\dot{v}\left(\frac{3x^2}{L^2} - \frac{2x^3}{L^3}\right)\right] = \underline{f(x,t)}$$
  

$$w(0,t) = 0; w'(0,t) = 0$$
  

$$w(1,t) = 0; w'(0,t) = 0$$
  

$$w(x,0) = -h(x)v(0)$$
  

$$w(x,0) = -h(x)\dot{v}(0)$$

#### Alternative approach for steady state response analysis



$$EIy^{iv} + m\ddot{y} + c\dot{y} = 0$$
  

$$y(0,t) = 0; y'(0,t) = 0$$
  

$$y(l,t) = \exp(i\omega t); y'(l,t) = 0$$
  

$$y(x,0) = 0; \dot{y}(x,0) = 0$$

$$EIy^{iv} + m\ddot{y} + c\dot{y} = 0$$

$$y(x,t) = \phi(x)\exp(i\omega t)$$

$$\Rightarrow$$

$$EI\phi^{iv} - m\omega^{2}\phi + i\omega c\phi = 0$$

$$\phi(0) = 0; \phi'(0) = 0; \phi(1) = 1; \phi'(1) = 0$$

$$\Rightarrow$$

$$\phi^{iv} - \lambda^{4}\phi = 0; \lambda^{4} = \frac{m\omega^{2} - i\omega c}{EI}$$

$$\phi(x) = a(\cos \lambda x + \cosh \lambda x) + b(\cos \lambda x - \cosh \lambda x)$$

$$+c(\sin \lambda x + \sinh \lambda x) + d(\sin \lambda x - \sinh \lambda x)$$

$$\phi'(x) = a\lambda(-\sin \lambda x + \sinh \lambda x) + b\lambda(-\sin \lambda x - \sinh \lambda x)$$

$$+c\lambda(\cos \lambda x + \cosh \lambda x) + d\lambda(\cos \lambda x - \cosh \lambda x)$$

$$\phi(0) = 0; \phi'(0) = 0; \phi(l) = 1; \phi'(l) = 0$$
  

$$\phi(x) = a(\cos \lambda x + \cosh \lambda x) + b(\cos \lambda x - \cosh \lambda x)$$
  

$$+c(\sin \lambda x + \sinh \lambda x) + d(\sin \lambda x - \sinh \lambda x)$$
  

$$\phi'(x) = a\lambda(-\sin \lambda x + \sinh \lambda x) + b\lambda(-\sin \lambda x - \sinh \lambda x)$$
  

$$+c\lambda(\cos \lambda x + \cosh \lambda x) + d\lambda(\cos \lambda x - \cosh \lambda x)$$
  

$$\phi(0) = 0 \Rightarrow a = 0$$
  

$$\phi'(0) = 0 \Rightarrow c = 0$$
  

$$\phi'(0) = 0 \Rightarrow c = 0$$
  

$$\phi(l) = 1 \Rightarrow b(\cos \lambda l - \cosh \lambda l) + d(\sin \lambda l - \sinh \lambda l)$$
  

$$\phi'(l) = 0 \Rightarrow b\lambda(-\sin \lambda l - \sinh \lambda l) + d\lambda(\cos \lambda l - \cosh \lambda l) = 0$$
  

$$b \& d \text{ can thus be determined.}$$
  

$$\Rightarrow y(x,t) = \left[b(\cos \lambda x - \cosh \lambda x) + d(\sin \lambda x - \sinh \lambda x)\right] \exp(i\omega t)$$

#### Random support motions



PSD function of 
$$y(x,t)$$
  
 $S_{YY}(x,\omega) = \lim_{T \to \infty} \frac{1}{T} \langle Y_T(x,\omega) Y_T^*(x,\omega) \rangle$   
 $Y_T(x,\omega) = \phi(x,\omega) V_T(\omega)$   
 $\Rightarrow$   
 $S_{YY}(x,\omega) = |\phi(x,\omega)|^2 S_{VV}(\omega)$ 

### **Merits of studying continuous systems**

•Means of first cut models for tall buildings, soil layers and line like structures such as chimneys and towers.

•For certain problems, continuous models may simplify the problem: for example, continuous models for lattice structures such as towers.

•If loads are rapidly fluctuating or when high frequency vibration is of interest it may be preferable to use continuous models.

•Exact solutions are possible for a class of problems. This is of educational value and also helps in assessing approximate methods of analysis.

## Limitations

- For each structural type, we need to develop a separate theory (axially vibrating bar, beam, arch, plate, shell...).
- Built-up structures (like building structures or water tanks) are difficult (if not impossible) to study using continuous system models.